

Results of ...

Phase IV - B Hydrogeological Investigation

for ...



**General Motors Corporation
CPC Group
Grand Rapids, Michigan**

July, 1988

20775

EDI Engineering & Science
Environmental Engineering, Geology, Biology and Chemistry
Grand Rapids, MI, (616) 942-0970



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SOURCE AREA GROUNDWATER REPORT PHASE IV-B

1.0 INTRODUCTION

Previous work in the former degreaser area at Column T-27 has delineated the extent of groundwater contamination from trichloroethylene (TCE) (See EDI report "Phase III-A Hydrogeological Investigation" dated March, 1987). The current phase of work was undertaken to assess the potential for removal of TCE from the groundwater and the control of groundwater flow in the source area to lessen the impact of the TCE source on the groundwater.

2.0 SCOPE OF WORK

The original scope of work included the development and an aquifer test of well 87-5 with monitoring of the nearby wells (Figure 1).

The scope of work was modified because of the poor yield of well 87-5 even after considerable development. The pumping test of well 87-5 was abandoned. Instead, mini-pump tests of wells 86-2, 87-1, 87-4, and 87-5 were conducted to determine the formation characteristics in those locations and to determine if adequate source area groundwater recovery and control was possible using these existing wells.

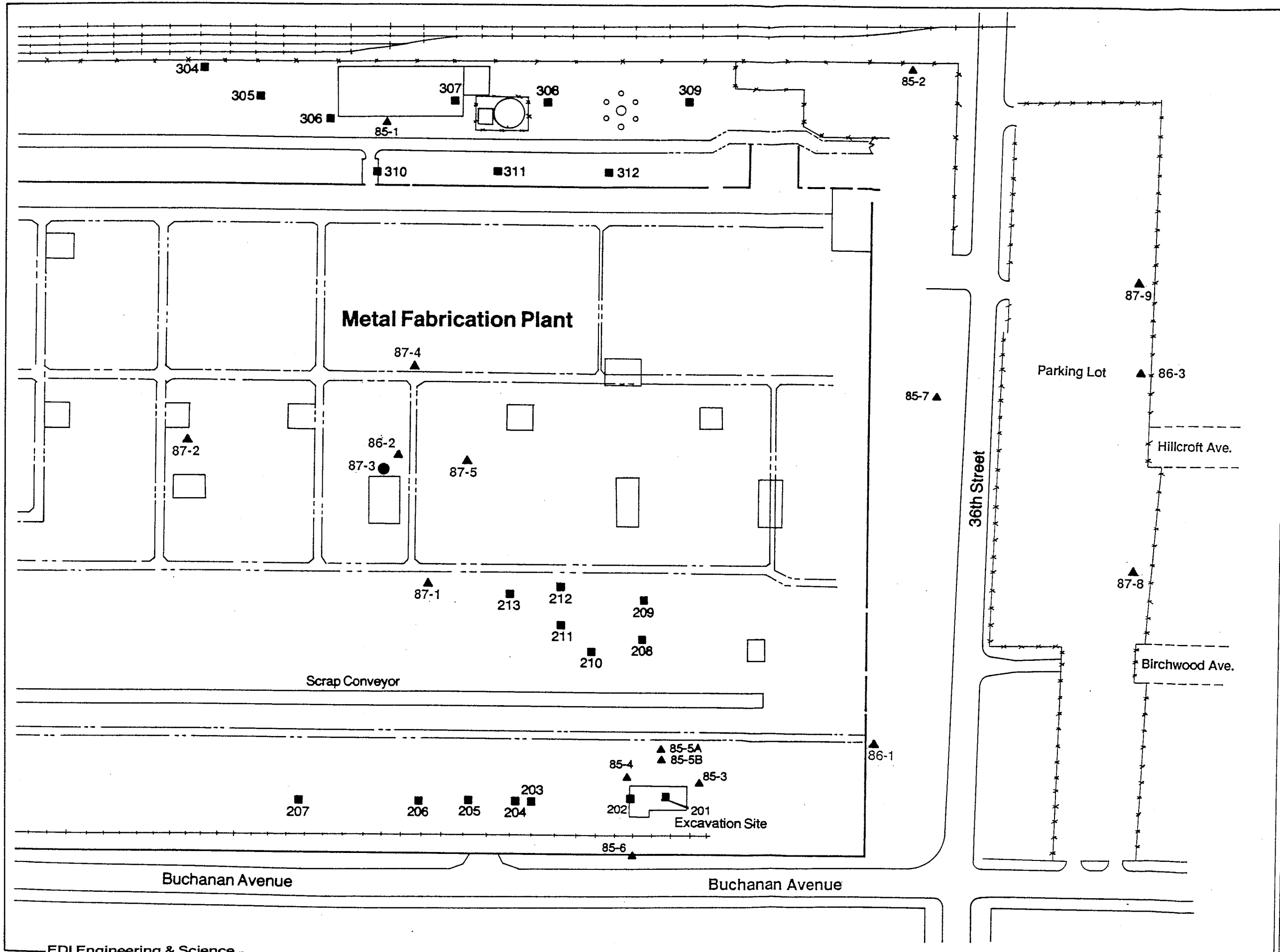
3. METHODS

The development work on well 87-5 was completed using CME 45 drilling equipment. Water levels were measured with a wetted tape. The development techniques are more fully described in the development section below and in Appendix A.

The mini-pump tests were conducted using an air-driven Isco bladder pump and *In-situ* Hermit data logger for recording groundwater levels. A wetted steel tape was used to reference the data logger. The groundwater recovered was pumped to barrels temporarily and transferred to the sanitary sewer at the steam-clean booth at column O-27.

The model of the influence of well 86-2 was constructed using a two-dimensional finite element numerical computer program.¹

(¹Townley, Lloyd R. and John L. Wilson, Description and User's Manual for a Finite Element Aquifer Flow Model Aquifer-1, Technology Adaptation Program Report No. 79-3, Massachusetts Institute of Technology, 1980.)



- Legend**
- ▲ Existing Monitoring Wells
 - Soil Vapor Well
 - Soil Boring Drilled By Soils and Materials Engineers March, 1985

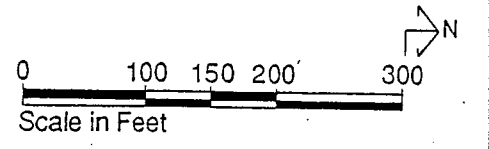


Figure 1
Well Locations
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 Grand Rapids, Michigan
 December, 1987 20775

4.0 DEVELOPMENT OF WELL 87-5

4.1 PREVIOUS WORK

Well 87-5 was set with 4 inch casing during the Phase III-A Investigation in anticipation that it was in the right place to intercept water from the source area, if necessary. The aquifer was sampled throughout its thickness with temporary wells set through 6-inch OD hollow stem augers. The augers were extended into the confining clay beneath the aquifer to confirm aquifer thickness. The permanent 4-inch well was set at its intended depth through the same boring using 10-inch OD hollow stem augers. The well was not developed when it was set because special procedures were required due to an additional upper screen set in the unsaturated soils. When the well was purged and sampled, the apparent yield of the well was less than 1 GPM. The expected pumping rate was 30 to 50 GPM. Our initial assessment of the low yield was that clay dragged upward by the augers coated the walls of the borehole.

4.2 CURRENT WORK

A considerable effort was made during the current phase of work to develop well 87-5 as a significant purge well for groundwater control of the source area. A summary and the field notes are included in Appendix A. The well was developed using several techniques.

In the first method, tri-sodium phosphate (TSP) was introduced to the well and allowed to react to disperse clay particles. Then, a rod pump was installed to remove water, clay and fine sand. The sequence of steps was repeated several times.

In the second method, TSP was again introduced to the well and allowed to react. Then surge blocks were installed in the well to push water through the screen in both directions. Finally, a submersible pump was used to remove water, clay, and fine sand. The sequence was repeated several times.

In the third method, completed later during the mini-pump tests, sulfamic acid was introduced to the well, allowed to react for two hours, and the well pumped with a bladder pump.

During the first two development sessions, significant clay and fine sand was recovered from the well. Clay and fine sand equivalent to 20% of the borehole volume adjacent to the screen was removed. The sustainable pumping rate increased only to 2 GPM. By the

end of the second session, the groundwater recovered no longer contained significant quantities of clay. Further development would not change the yield of the well. The acid treatment of the well caused additional fine sand to be removed via the bladder pump. However, the sustainable pumping rate did not significantly improve.

The lack of significant increase in specific yield of the well during development despite removal of significant quantities of clay and fine sand indicates that the formation surrounding the well is less permeable than would be estimated from the original log. Continued development of the well is not warranted. A small increase in yield is all that could be expected and would not provide a significant recovery well for purging contaminated groundwater and for control of the groundwater flow in the source area.

5.0 MINI PUMP TESTS

The water elevations resulting from pumping well 86-2 with an air-driven bladder pump are listed in Appendix B. Figure 2 is a graph of the water elevation versus time. The surging characteristic of the bladder pump is evident from the graph. The overall trend, however, showed the water level stabilizing very quickly in the well. Comparison of the drawdown curve with the recovery curve plotted in Figure 3 shows a very similar and rapid response to the change in water level. The hydraulic conductivity of the aquifer at well 86-2 had been previously calculated to be $K=30$ feet/day. Comparison in Figure 4 of the rate of stabilization for the conductivity test and the recovery from the pump test shows the recovery of the pump test to be slower. The well had not been pumped previous to the mini-pump test during the last year. Tests of hydraulic conductivity for non-pumping wells sometimes show a decrease in hydraulic conductivity over time. When these wells are pumped or redeveloped the response rate will recover at least partially.

The well was pumped at 3/4 GPM for two hours to determine if the pumping rate is sustainable. The water level stabilized and within the accuracy allowed by the bladder pump surge shows a slight trend upward in water level during the test. The slight upward trend may indicate the well was regaining some of its response rate. The drawdown created by the 3/4 GPM pumping rate was .45 ft as measured near the beginning of the pump test before the upward trend in water level is apparent. The conservatively calculated specific capacity of Well 86-2 is thus 1.67 GPM/ft. The top of the well screen is set 5.5 feet below the water table and the screen extends an additional five feet into the groundwater. The maximum long term pumping rate is that obtainable by pulling the

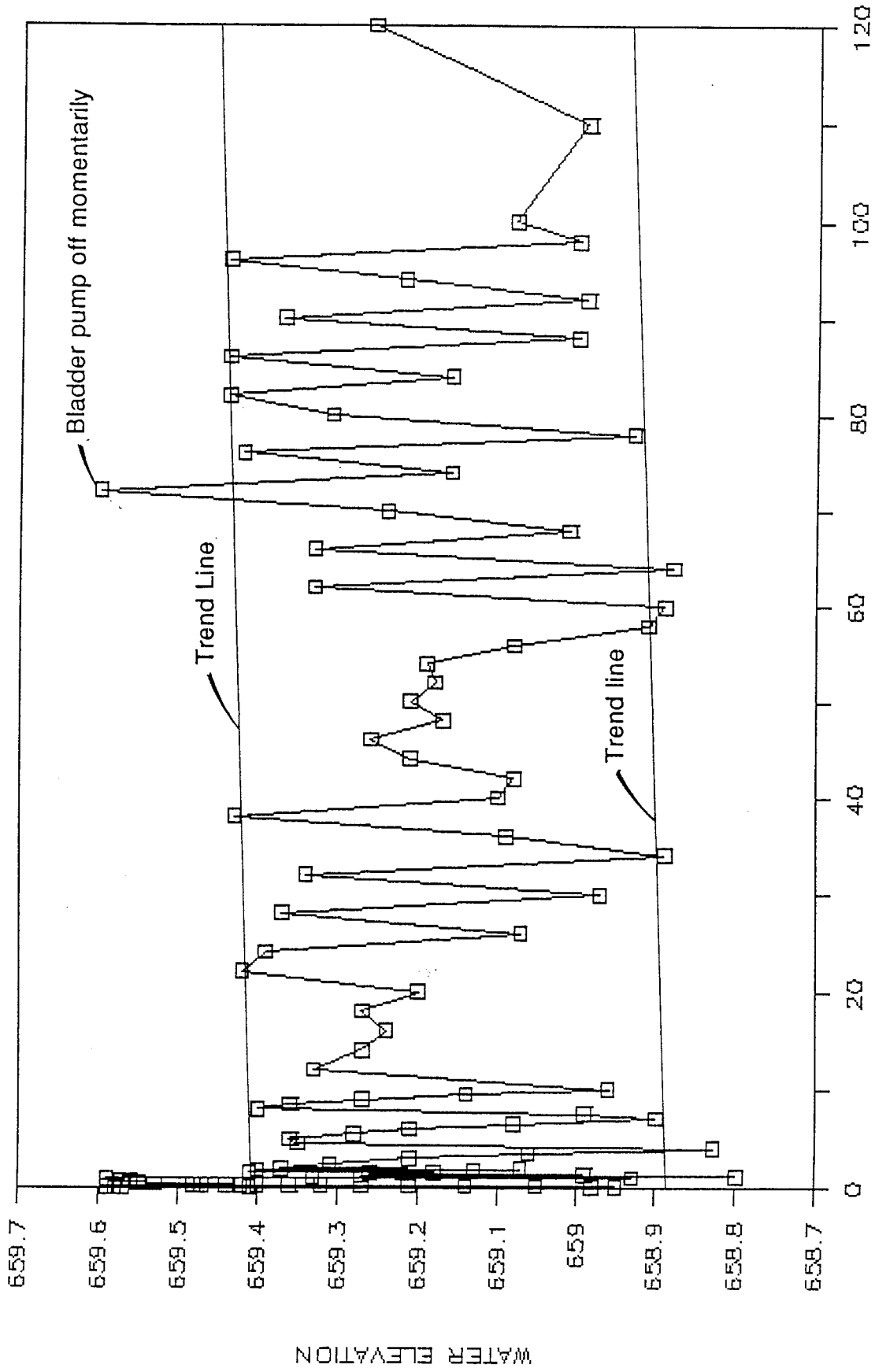
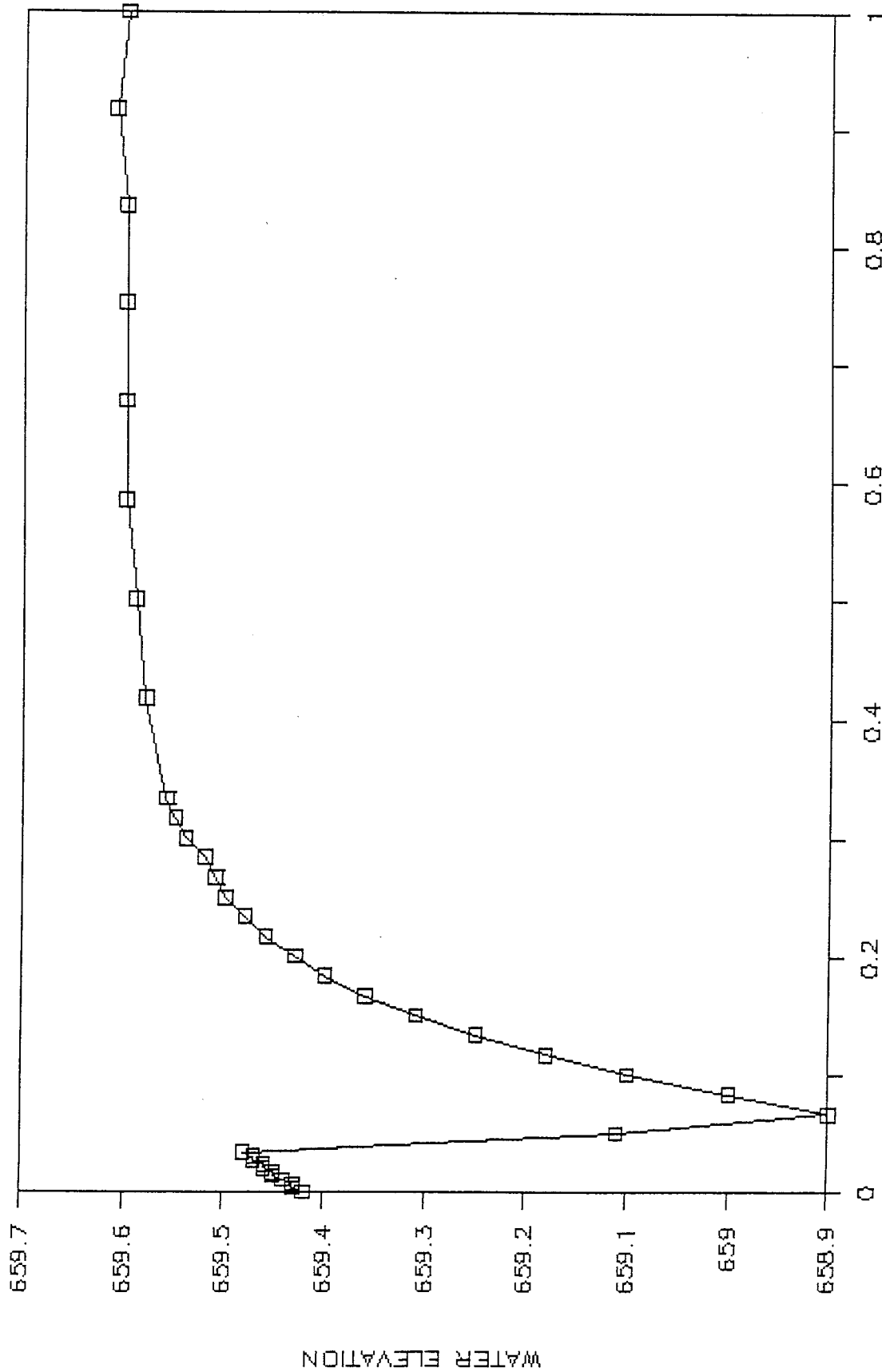


Figure 2
CPC Source Area Pump Tests
Well 86-2 9/23/87 3/4 GPM
 General Motors Corp./CPC Group
 Grand Rapids, Michigan
 July, 1988



□ RECORDED WATERLEVEL
 (Note Time Scale Change from Figure 2)

Figure 3
 CPC Source Area Pump Tests
 Well 86-2 9/23/87 Recovery
 General Motors Corp./CPC Group
 Grand Rapids, Michigan
 July, 1988
 20775

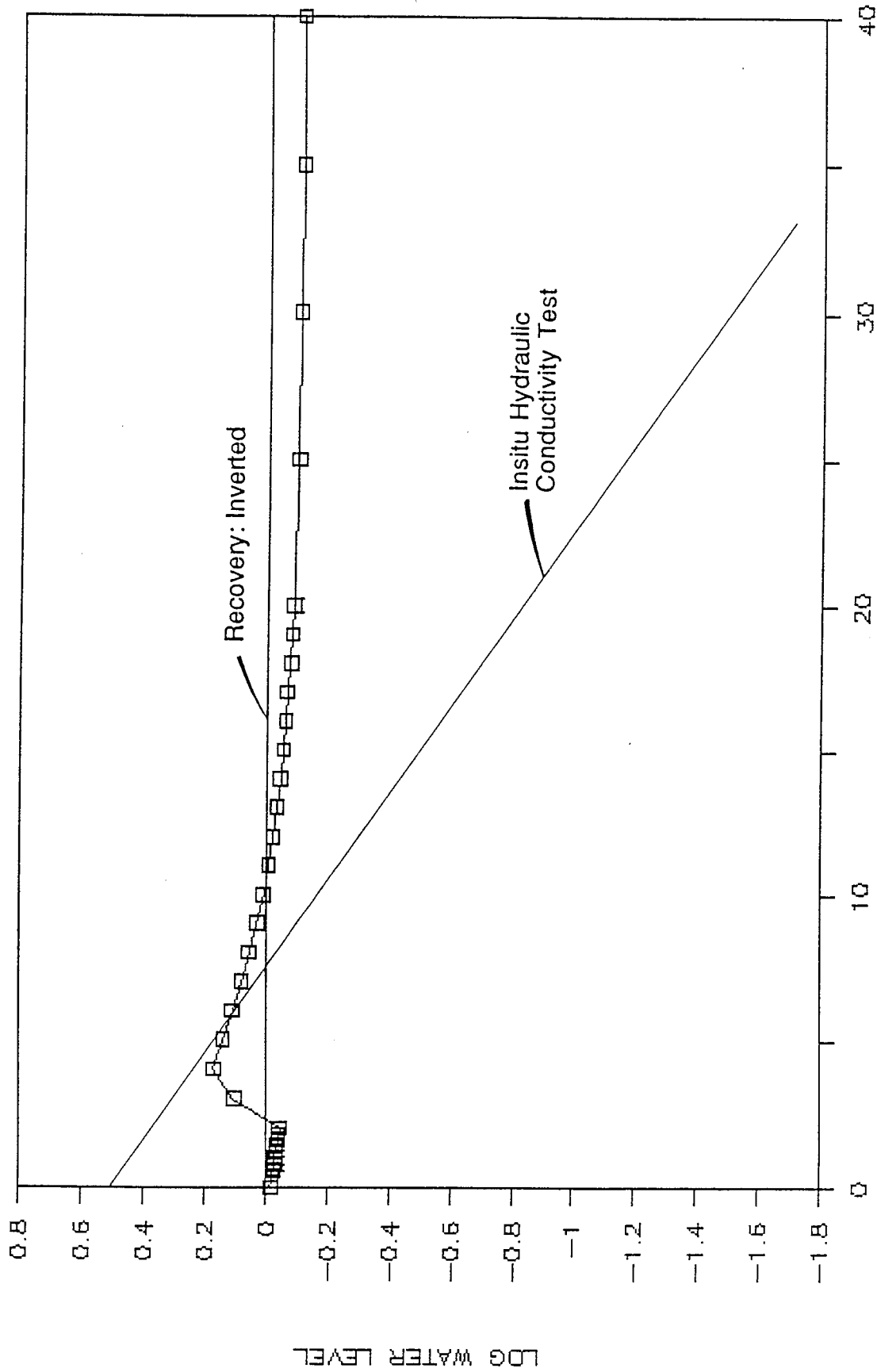


Figure 4
CPC Well 86-2 Comparison
9/87 Recovery VS 5/86 Conductivity
 General Motors Corp./CPC Group
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 July, 1988
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water level at the well down to the top of the screen. The calculated sustainable pumping rate of the well would be $1.67 \text{ GPM/ft} \times 5.5\text{ft} = 9 \text{ GPM}$. For long term operating purposes, the pumping rate should be considered as 8 GPM. If the pumping of the well enhances the redevelopment of the well, the ultimate rate of sustainable pumping may be 10 GPM.

Well 86-2 is located immediately adjacent to the suspected source area, so the pumping of the well will pull groundwater through the most contaminated zone of the aquifer. The pumping of well 86-2 will create a cone of depression in the groundwater around the well. Since the source area is located centrally within the plant, the normal rainfall recharge to this water table aquifer will not be present in the source area. Recharge to the groundwater in the source area is only available through the movement of water along the flow gradient in the aquifer. A numerical model was constructed of the drawdown created by the pumping of well 86-2 within the normal gradient of the aquifer beneath the plant. The normal gradient beneath the plant was simulated in this simplified model by fixing the water levels on the north and south sides of the model area with an appropriate elevation difference for the gradient. The distributions of water levels around the well were calculated using a range of hydraulic conductivity from 3 ft/day to 30 ft/day and pumping rates bracketing 8 GPM. Pumping rates were adjusted for the maintenance of five feet of drawdown in some trials. The resulting cone of depression of the well is on the order of 100ft to 200ft radius. Figure 5 shows the estimated cone of depression and areal influence of well 86-2. The areal influence of the well is that area from which groundwater flows into the well. It extends upgradient far beyond the source in the shape of a parabola with the focus at the well. Representative graphs of the model runs are included in Appendix C.

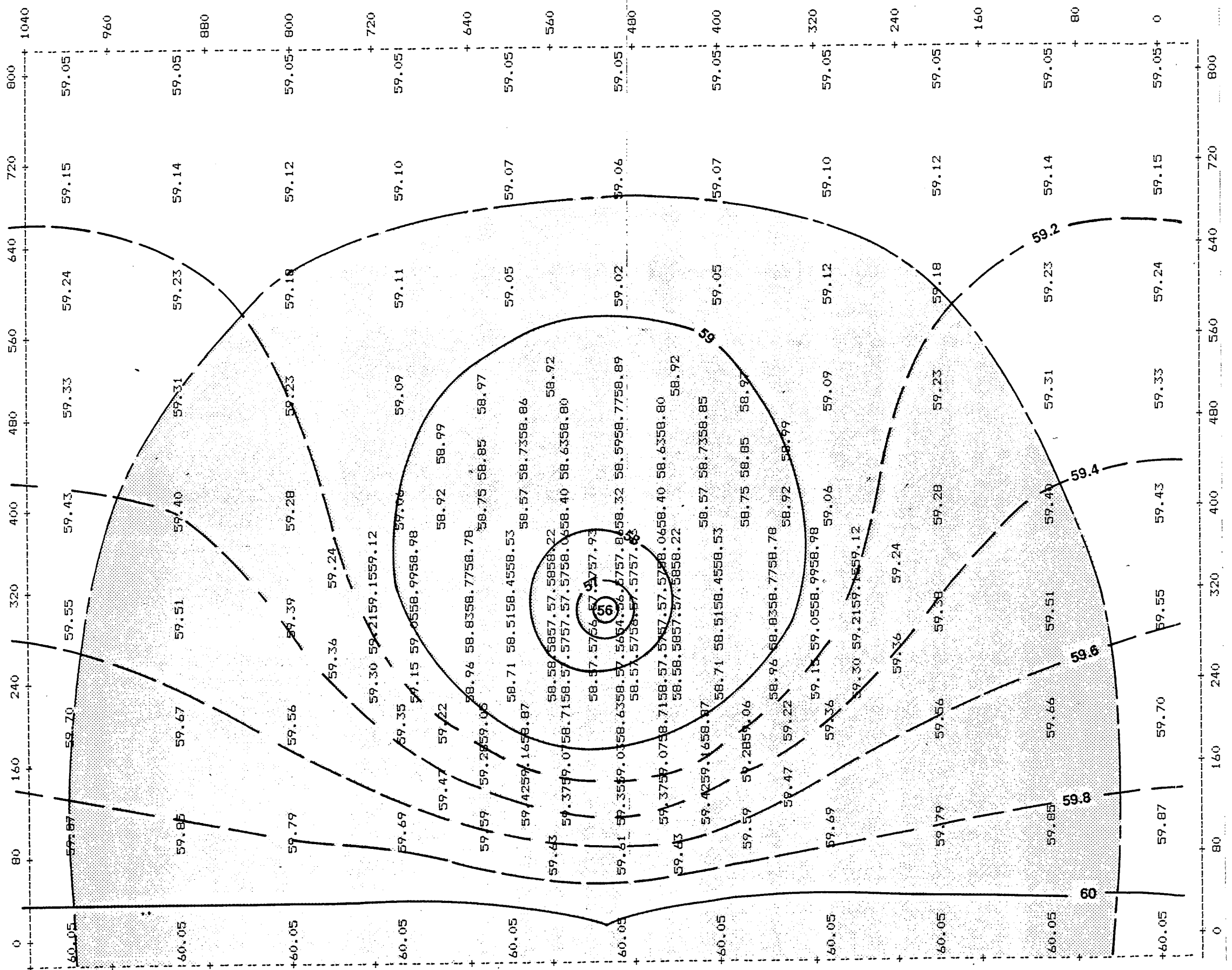
The other locations on site where hydraulic conductivity has been measured are a significant distance from the source area to the northeast, north, and northwest. The other locations all have higher values of hydraulic conductivity than does the source area. If the hydraulic conductivity of the aquifer in the source area increases rapidly away from well 86-2, the cone of depression of well 86-2 would be significantly reduced. The available information does not provide a definitive answer. Monitoring of the water levels in the plant wells and particularly the four wells around the source area will define the areal extent of the cone of depression if it extends at least 100 feet. If the cone is not defined at the nearby existing wells, two additional monitor wells nearer to well 86-2 would be useful for determining its area of influence.

The pumping of well 86-2 will flush the groundwater in the source area within its capture zone. The capture zone of the well is that area defined by both the areal influence of the well and the source area contamination. Approximately 20,000 square feet are in the capture zone. The volume of water in the aquifer in the capture zone would be the area in the capture zone X aquifer thickness X aquifer porosity = 20,000 sq ft X 25 ft X .25 = 125,000 cu ft. The well will remove 1540 cu ft/day at 8 gpm, so removal of one capture zone volume of water will take approximately 80 days.

The concentration of TCE measured in well 86-2 previously was 90 mg/l. Calculation of the rate of removal of TCE at 8 GPM yields .35 lb/hr or 8.4 lb/day. If the pumping of well 86-2 removes the volume of the capture zone in approximately 80 days, then the concentration of TCE would decrease over a time period of a few months (three volumes of the capture zone) to approximately half its previously measured concentration. Because of the likely circumstance that residual TCE liquids are in the soils below the water table, the concentration of TCE in the groundwater reaching well 86-2 should stabilize until the residuals are depleted. The removal rate of TCE should still be on the order of 4 lb/day. Operation of a pumping system in well 86-2 will remove a significant amount of TCE from the source area.

Wells 87-1 and 87-4 are two inch wells set east and west of the source area respectively. These wells were constructed with an upper screen in the soils above the water table for use with the soil vapor removal system. Because of the upper screens, the use of these wells for pumping of groundwater has to be approached in a different manner. The bladder pump could not be lowered completely through the upper screen. The lower portion of the bladder pump was submerged in the groundwater and the pump was operated at 1/2 GPM for a period of 1-1/2 hours on each well. The water level was not lowered below the level of the pump in either well during the test period. The top of the well screen is two feet below the surface of the groundwater in both wells 87-1 and 87-4. We estimate that 2 GPM is the maximum sustainable pumping rate for each well.

Well 87-5 was pumped with the bladder pump to determine its specific capacity following treatment with acid. Figure 5 is the drawdown caused by pumping at 1 GPM, and Figure 6 is the recovery of the well. The estimated specific capacity of the well is 0.10 GPM/ft. With approximately 20 feet of drawdown available to the top of the well screen, the sustainable pumping rate would be 2 GPM.



Legend

- Water Level Contour Line
- ▨ Zone of Influence

Aquifer Well Drawdown Model:
 Pumping well 86-2 at 8gpm K=6
 Time = 20.00000
 X Coordinates Range from .00 to 800.00
 Y Coordinates Range from .00 to 1000.00
 Scale: 1/3000.

Figure 5

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 Grand Rapids, Michigan

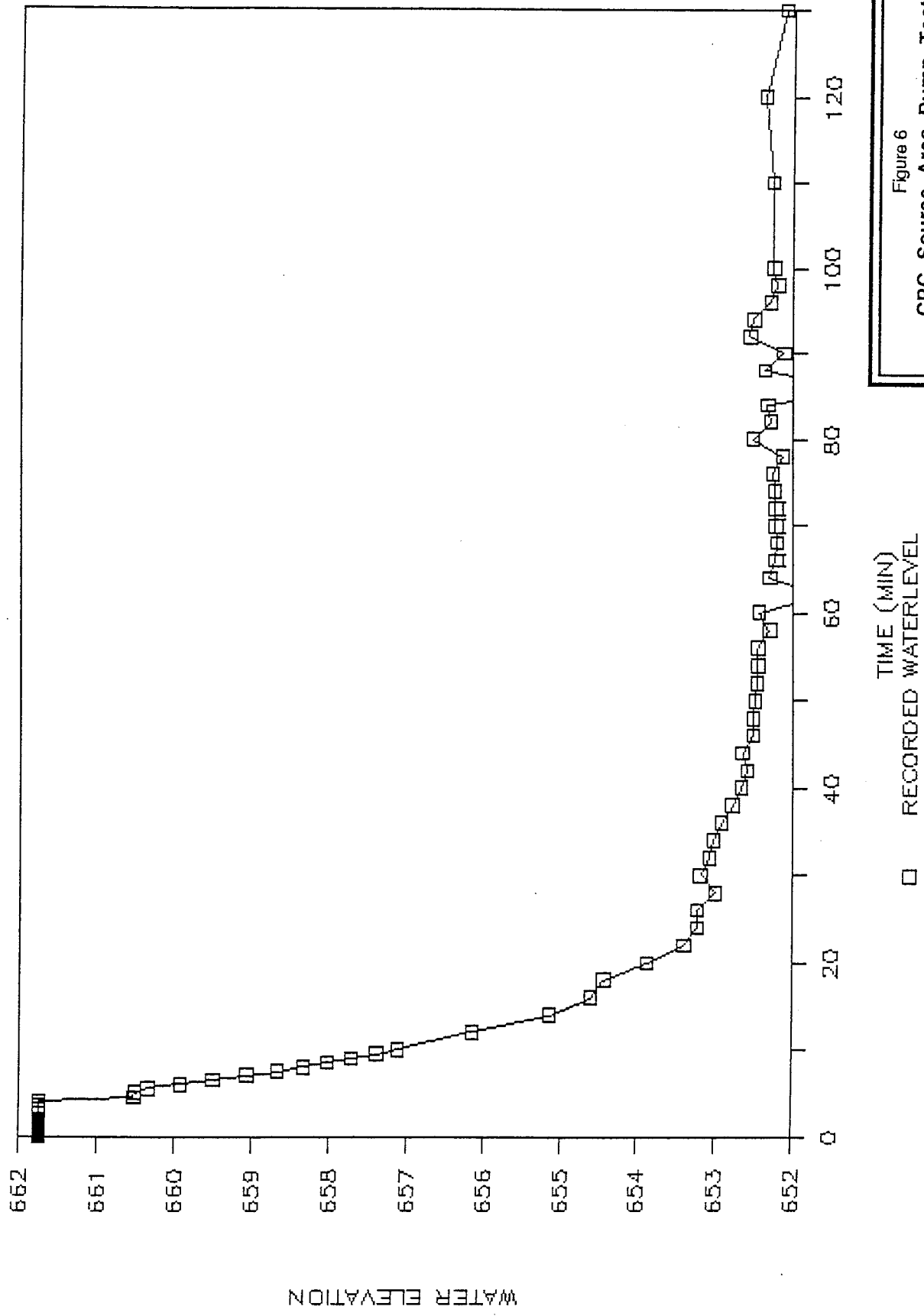


Figure 6
CPC Source Area Pump Tests
Well 87-5 9/24/87 1 GPM
 General Motors Corp./CPC Group
 Grand Rapids, Michigan

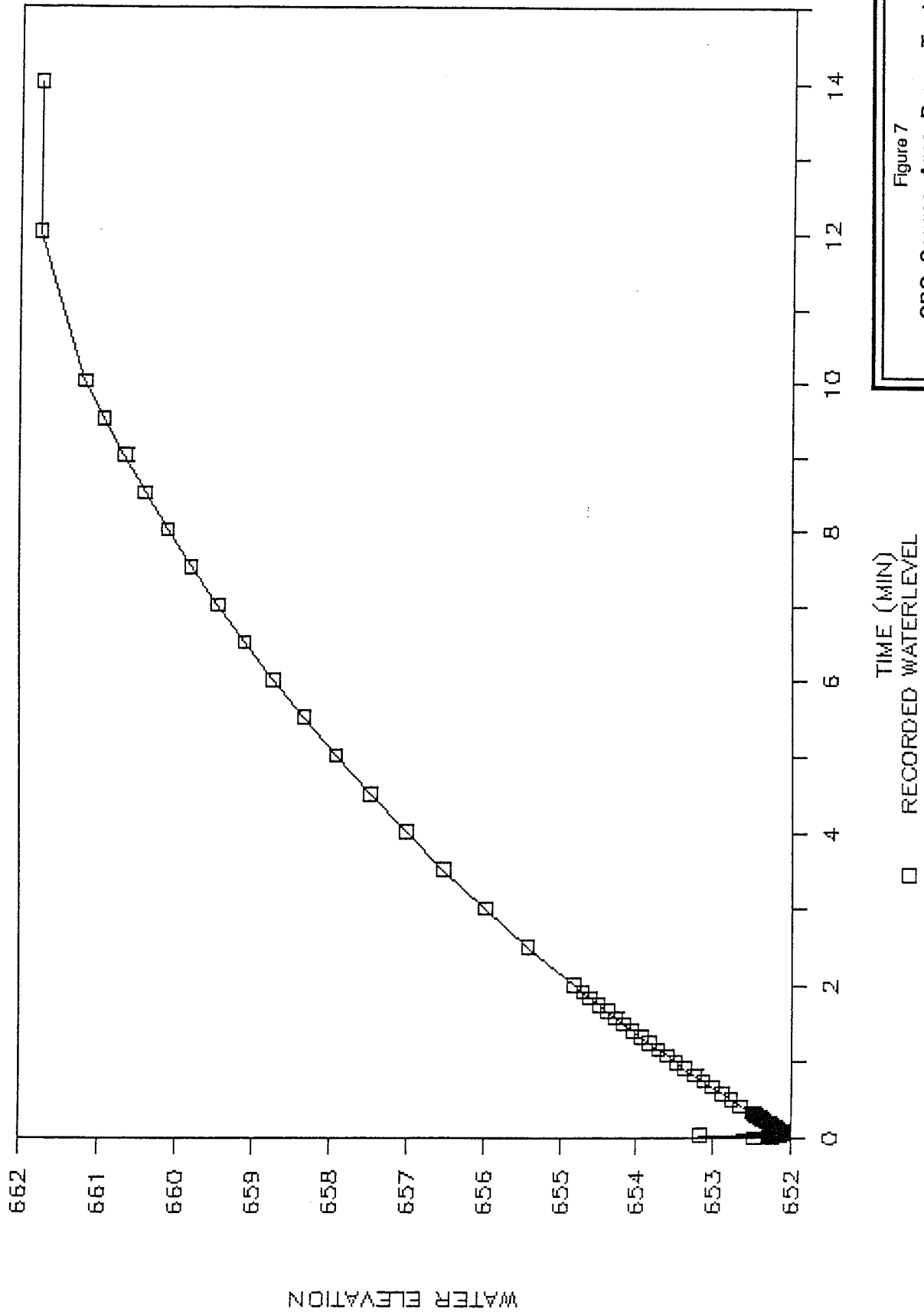


Figure 7

CPC Source Area Pump Tests
 Well 87-5 9/24/87 Recovery
 General Motors Corp./CPC Group
 Grand Rapids, Michigan

July, 1988

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6.0 CONCLUSIONS

1. Additional development of well 87-5 is unwarranted. The well can be used for monitoring purposes but is not suitable for use as a groundwater purge well.
2. Well 86-2 should be used to recover groundwater with high levels of TCE. The rate of recovery would initially be approximately 8 lb/day. The concentrations will decrease over the time required to remove three volumes from the capture zone. Assuming the presence of residual liquids, an estimate of longer term recovery of TCE would be 4 lb/day. This estimate represents the high side of expected long-term recovery rates.
3. The zone of influence of well 86-2 appears to cover a significant portion of the source area, based on a simplified model of the site. Pumping of the well will impede the flow of TCE out of the source area.

7.0 RECOMMENDATIONS

1. Install a shallow well jet pump in well 86-2 to recover ground water at 8 to 10 GPM.
2. Treat the ground water with steam stripping or aqueous carbon adsorption depending upon the cost effectiveness of the alternatives.
3. Discharge the treated water to the sanitary sewer with TOX concentrations not to exceed 2.1 ppm, total.
4. Evaluate the performance of the ground water recovery system during the first thirty days of operation to 1) determine the effectiveness of the well in controlling the source area ground water, and 2) estimate the trend of the ground water TCE concentrations.
5. Install two additional shallow monitoring wells in the source area if the cone of depression of well 86-2 cannot be determined accurately by the other existing monitoring wells.

APPENDIX A

**Summary for Development of
Well 87-5**

DEVELOPMENT OF WELL 87-5

The following is a summary of the field work accomplished.

Monday, August 31, 1987

Previous attempts at pumping the well gave a yield of 1 gpm with submersible pump at a pwl of 50 feet.

Four-and-one-half pounds of TSP was pumped down the well which had a 30-foot water column. Pumping was then started with a 4-inch plunger. Volume was unable to be determined because water ran out of the upper screen. Pumping continued for 15 minutes. The plunger was then pulled and the well head closed.

Tuesday, September 1, 1988

1. Installed 2-inch casing and screen and then rod pumped at a rate of 1 gpm which resulted in very muddy water.
2. A 4-inch plunger pump was then installed, followed by the installation of a 2- by 4-inch K-packer above and below the upper screen. Pumping at 1 gpm resulted in muddy water.
3. Four-and-one-half pounds TSP was added and allowed to sit for 2 hours. The well pressurized with fresh water, 200 gallons backwash pumped with submersible pump at 1 gpm, backwash 100 gallons of water at 2.5 gpm.

September 3, 1987

1. Four-and-one-half pounds TSP was added.
2. A K-packer was installed to split the 10-foot screen into two sections for development surging. "A" is the lower section; "B" is the upper section.
3. Surge "A" for 40 minutes. Surge "B" for 20 minutes.
4. Pumped 70 gallons of muddy water with submersible pump (trace of sand).
5. Four-and-one-half pounds TSP was added.
6. Surge "A" for 30 minutes. Surge "B" for 30 minutes.
7. Pumped 50 gallons of cloudy water then 20 gallons of semi-cloudy water and 7 cc sand/liter.

September 4, 1987

1. Surge "A" for 40 minutes. Surge "B" for 40 minutes.
2. Pumped 30 gallons of cloudy water, then semi-cloudy, then semi-clear and 2 cc sand/liter; then clear and sand-free.
3. Four pounds TSP was added.
4. Surge "A" for 60 minutes. Surge "B" for 60 minutes.
5. Pumped 60 gallons of muddy water, then 20 gallons of cloudy water, then 20 gallons clear with some sand.

September 8, 1987

1. Four pounds TSP was added. Surged "A" 45 minutes (9-9:45).
2. Four pounds TSP was added and 2 gallons of water. Surged "B" 55 minutes (10:05-11:00).
3. Pumped 20 gallons in two minutes; very cloudy with fine sand. Flow rate then dropped to 1-3/4 gpm. Purged 15 minutes; cloudy to semi-cloudy, loss of sand. Sand cone 0.3 cc sand after settling 10 minutes.
4. Added 4 pounds TSP with 2-2.5 gallons of water. Surged "A" 30 minutes (12:30-1:00).
5. Added 4 pounds TSP with 2 gallons of water. Surged "B" 25 minutes (1:20-1:45).
6. Purged 20 gallons at 12 gpm with submersible pump; very cloudy with sand. After approximately 2 minutes dropped rate to 1-3/4 gpm. Purged at 1-3/4 for 15 minutes; sand cone 0.5 cc/liter. After 25 minutes, purging little to no sand.

APPENDIX B
Pump Test Data

Well 86-2

Drawdown		1.5000	22.12	84.0000	22.22
SE10008		1.5833	22.20	86.0000	21.94
Environmental Logger		1.6667	21.97	88.0000	22.38
09/23 17:09		1.7500	22.25	90.0000	22.01
		1.8333	21.98	92.0000	22.39
Unit# 00004 Test# 0		1.9167	22.31	94.0000	22.16
		2.0000	22.01	96.0000	21.94
INPUT 1: Level (F) TOC		2.5000	22.07	98.0000	22.38
		3.0000	22.17	100.0000	22.30
Reference	21.81	3.5000	22.32	110.0000	22.39
Scale factor	10.09	4.0000	22.55	120.0000	22.12
Offset	0.00	4.5000	22.03		
		5.0000	22.02	END	
Step# 0	09/23 13:16	5.5000	22.10		
		6.0000	22.17		
Elapsed Time	Value	6.5000	22.30		
-----	-----	7.0000	22.48		
0.0000	21.81	7.5000	22.39		
0.0033	21.81	8.0000	21.98		
0.0066	21.80	8.5000	22.02		
0.0099	21.80	9.0000	22.11		
0.0133	21.80	9.5000	22.24		
0.0166	21.80	10.0000	22.42		
0.0200	21.80	12.0000	22.05		
0.0233	21.80	14.0000	22.11		
0.0266	21.79	16.0000	22.14		
0.0300	21.80	18.0000	22.11		
0.0333	21.80	20.0000	22.18		
0.0500	21.79	22.0000	21.96		
0.0666	21.79	24.0000	21.99		
0.0833	21.79	26.0000	22.31		
0.1000	21.79	28.0000	22.01		
0.1166	21.97	30.0000	22.41		
0.1333	22.40	32.0000	22.04		
0.1500	22.43	34.0000	22.49		
0.1666	22.33	36.0000	22.29		
0.1833	22.24	38.0000	21.95		
0.2000	22.17	40.0000	22.28		
0.2166	22.11	42.0000	22.30		
0.2333	22.06	44.0000	22.17		
0.2500	22.02	46.0000	22.12		
0.2666	21.98	48.0000	22.21		
0.2833	21.96	50.0000	22.17		
0.3000	21.94	52.0000	22.20		
0.3166	21.91	54.0000	22.19		
0.3333	21.90	56.0000	22.30		
0.4167	21.85	58.0000	22.47		
0.5000	21.83	60.0000	22.49		
0.5833	21.82	62.0000	22.05		
0.6667	21.82	64.0000	22.50		
0.7500	21.81	66.0000	22.05		
0.8333	21.81	68.0000	22.37		
0.9167	21.79	70.0000	22.14		
1.0000	22.45	72.0000	21.78		
1.0833	22.05	74.0000	22.22		
1.1667	22.58	76.0000	21.96		
1.2500	22.11	78.0000	22.45		
1.3333	22.39	80.0000	22.07		
1.4166	22.16	82.0000	21.94		

Well 86-2 Recovery

SE10008		1.5000	21.78
Environmental Logger		1.5833	21.77
09/23 17:12		1.6667	21.77
		1.7500	21.77
		1.8333	21.77
Unit# 00004 Test# 0		1.9167	21.78
		2.0000	21.78
INPUT 1: Level (F) TOC		2.5000	21.77
		3.0000	21.77
Reference 21.81		3.5000	21.77
Scale factor 10.09		4.0000	21.77
Offset 0.00		4.5000	21.77
		5.0000	21.77
Step# 1 09/23 15:19		5.5000	21.77
		6.0000	21.77
Elapsed Time Value		6.5000	21.77
-----		7.0000	21.77
0.0000 21.96		7.5000	21.77
0.0033 21.95		8.0000	21.77
0.0066 21.95		8.5000	21.77
0.0099 21.94		9.0000	21.77
0.0133 21.93		9.5000	21.77
0.0166 21.93		10.0000	21.77
0.0200 21.92		12.0000	21.77
0.0233 21.92		14.0000	21.77
0.0266 21.91		16.0000	21.77
0.0300 21.91		18.0000	21.77
0.0333 21.90		20.0000	21.77
0.0500 22.27		22.0000	21.77
0.0666 22.48		24.0000	21.77
0.0833 22.38		26.0000	21.77
0.1000 22.28		28.0000	21.77
0.1166 22.20		30.0000	21.77
0.1333 22.13		32.0000	21.77
0.1500 22.07		34.0000	21.76
0.1666 22.02		36.0000	21.75
0.1833 21.98		38.0000	21.76
0.2000 21.95		40.0000	21.76
0.2166 21.92		42.0000	21.76
0.2333 21.90			
0.2500 21.88		END	
0.2666 21.87			
0.2833 21.86			
0.3000 21.84			
0.3166 21.83			
0.3333 21.82			
0.4167 21.80			
0.5000 21.79			
0.5833 21.78			
0.6667 21.78			
0.7500 21.78			
0.8333 21.78			
0.9167 21.77			
1.0000 21.78			
1.0833 21.77			
1.1667 21.77			
1.2500 21.78			
1.3333 21.78			
1.4166 21.77			

Well 87-5 Drawdown

SE1000B		1.5000	26.59
Environmental Logger		1.5833	26.49
09/25 08:50		1.6667	26.38
		1.7500	26.27
Unit# 00004 Test# 0		1.8333	26.16
		1.9167	26.06
		2.0000	25.95
INPUT 1: Level (F) TOC		2.5000	25.35
		3.0000	24.79
Reference 19.03		3.5000	24.25
Scale factor 10.09		4.0000	23.77
Offset 0.00		4.5000	23.30
		5.0000	22.85
Step# 1 09/24 15:19		5.5000	22.44
		6.0000	22.04
Elapsed Time Value		6.5000	21.67
-----	-----	7.0000	21.32
0.0000	28.29	7.5000	20.98
0.0033	28.51	8.0000	20.67
0.0066	28.48	8.5000	20.38
0.0099	28.49	9.0000	20.11
0.0133	28.48	9.5000	19.85
0.0166	28.48	10.0000	19.60
0.0200	28.48	12.0000	19.03
0.0233	28.47	14.0000	19.03
0.0266	28.46	16.0000	19.03
0.0300	28.62	18.0000	19.03
0.0333	27.59	20.0000	19.03
0.0500	28.54	22.0000	19.03
0.0666	28.64	24.0000	19.03
0.0833	28.64	26.0000	19.03
0.1000	28.61	28.0000	19.03
0.1166	28.59	30.0000	19.03
0.1333	28.57		
0.1500	28.53	END	
0.1666	28.51		
0.1833	28.49		
0.2000	28.46		
0.2166	28.43		
0.2333	28.41		
0.2500	28.39		
0.2666	28.36		
0.2833	28.32		
0.3000	28.31		
0.3166	28.28		
0.3333	28.26		
0.4167	28.11		
0.5000	27.99		
0.5833	27.87		
0.6667	27.74		
0.7500	27.63		
0.8333	27.51		
0.9167	27.38		
1.0000	27.27		
1.0833	27.16		
1.1667	27.04		
1.2500	26.93		
1.3333	26.82		
1.4166	26.70		

Well 87-5 Recovery

		1.5000	19.03	84.0000	28.44
SE10008		1.5833	19.03	86.0000	29.79
Environmental Logger		1.6667	19.03	88.0000	28.40
09/25 08:47		1.7500	19.03	90.0000	28.64
		1.8333	19.03	92.0000	28.21
Unit# 00004 Test# 0		1.9167	19.03	94.0000	28.26
		2.0000	19.03	96.0000	28.48
INPUT 1: Level (F) TOC		2.5000	19.03	98.0000	28.56
		3.0000	19.03	100.000	28.51
Reference	19.03	3.5000	19.03	110.000	28.50
Scale factor	10.09	4.0000	19.03	120.000	28.40
Offset	0.00	4.5000	20.24	130.000	28.66
		5.0000	20.26		
Step# 0	09/24 13:07	5.5000	20.42	END	
		6.0000	20.85		
Elapsed Time	Value	6.5000	21.27		
-----	-----	7.0000	21.71		
0.0000	19.03	7.5000	22.11		
0.0033	19.03	8.0000	22.44		
0.0066	19.03	8.5000	22.75		
0.0099	19.03	9.0000	23.06		
0.0133	19.03	9.5000	23.38		
0.0166	19.03	10.0000	23.65		
0.0200	19.03	12.0000	24.63		
0.0233	19.03	14.0000	25.63		
0.0266	19.03	16.0000	26.17		
0.0300	19.03	18.0000	26.33		
0.0333	19.03	20.0000	26.88		
0.0500	19.03	22.0000	27.36		
0.0666	19.03	24.0000	27.53		
0.0833	19.03	26.0000	27.52		
0.1000	19.03	28.0000	27.76		
0.1166	19.03	30.0000	27.57		
0.1333	19.03	32.0000	27.68		
0.1500	19.03	34.0000	27.73		
0.1666	19.03	36.0000	27.84		
0.1833	19.03	38.0000	27.98		
0.2000	19.03	40.0000	28.09		
0.2166	19.03	42.0000	28.17		
0.2333	19.03	44.0000	28.11		
0.2500	19.03	46.0000	28.24		
0.2666	19.03	48.0000	28.24		
0.2833	19.03	50.0000	28.27		
0.3000	19.03	52.0000	28.30		
0.3166	19.03	54.0000	28.31		
0.3333	19.03	56.0000	28.31		
0.4167	19.03	58.0000	28.46		
0.5000	19.03	60.0000	28.32		
0.5833	19.03	62.0000	29.16		
0.6667	19.03	64.0000	28.46		
0.7500	19.03	66.0000	28.54		
0.8333	19.03	68.0000	28.55		
0.9167	19.03	70.0000	28.54		
1.0000	19.03	72.0000	28.54		
1.0833	19.03	74.0000	28.53		
1.1667	19.03	76.0000	28.50		
1.2500	19.03	78.0000	28.63		
1.3333	19.03	80.0000	28.24		
1.4166	19.03	82.0000	28.48		

APPENDIX C
Model Results and Plots

AQUIFEM-1
=====

TWO DIMENSIONAL FINITE ELEMENT MODEL
OF GROUNDWATER FLOW WITH
LINEAR TRIANGULAR ELEMENTS

DEPARTMENT OF CIVIL ENGINEERING
MASSACHUSETTS INSTITUTE OF TECHNOLOGY
CAMBRIDGE, MASSACHUSETTS. 02139

NOVEMBER 1979 VERSION

AQUIFEM WELL DRAWDOWN MODEL:pumping well Bp-2 at Bgpm LOWERED K

PROBLEM DESCRIPTION
=====

TYPE OF AQUIFER.....LINEAR= .F.
LINEAR=.T. LINEAR (CONFINED) OR LINEARIZED AQUIFER
LINEAR=.F. NONLINEAR (UNCONFINED OR MIXED) AQUIFER

TYPE OF PROBLEM.....STEADY= .F.
STEADY=.T. STEADY STATE PROBLEM
STEADY=.F. UNSTEADY OR TRANSIENT PROBLEM

INITIAL CONDITIONS/GUESSES PARAMETER.....INITIAL= .F.
INITIAL=.T. COMPUTE LINEAR SOLUTION AS A FIRST GUESS FOR A NONLINEAR PROBLEM
AND/OR INITIAL STEADY STATE SOLUTION FOR AN UNSTEADY PROBLEM
INITIAL=.F. READ INITIAL CONDITIONS OR GUESSES

FINAL STEADY STATE SOLUTION PARAMETER.....FINAL= .F.
FINAL=.T. COMPUTE FINAL STEADY STATE SOLUTION WITH BOUNDARY CONDITIONS AT TIME=ENDTIM
FINAL=.F. DO NOT COMPUTE FINAL STEADY STATE SOLUTION

DATA CHECK PARAMETER.....CHECK= .F.
CHECK=.T. CHECK DATA THEN STOP
CHECK=.F. EXECUTE PROGRAM NORMALLY

FINITE ELEMENT GRID PARAMETERS

=====

NUMBER OF ELEMENTS.....NUMEL= 408
 NUMBER OF NODE POINTS.....NUMNP= 223
 FLAG FOR PRINTING GEOMETRIC DATA.....
 PRGEOM=.T. PRINT GEOMETRIC DATA
 PRGEOM=.F. DO NOT PRINT GEOMETRIC DATA
 FLAG FOR PRINT-FITTING GEOMETRIC DATA.....
 FLGEOIN=.T. FLOT GEOMETRIC DATA
 FLGEOIN=.F. DO NOT FLOT GEOMETRIC DATA
 SCALING FACTOR FOR X-COORDINATES.....XSCALE= 10.000
 SCALING FACTOR FOR Y-COORDINATES.....YSCALE= 10.000
 LARGEST ELEMENT IDENTIFICATION NUMBER.....MAXELT= 408
 LARGEST NODE IDENTIFICATION NUMBER.....MAXNOD= 223

1
 AQUIFER PROPERTY PARAMETERS
 =====

NUMBER OF REGIONS WITH REGIONALLY DEFINED AQUIFER PROPERTIES.....NAOR= 0
 NUMBER OF INDIVIDUAL ELEMENTS OR NODES WITH SPECIALLY DEFINED AQUIFER PROPERTIES.....NSPEC= 0
 METHOD BY WHICH AQUIFER PROPERTIES ARE GIVEN.....
 BYNODE=.T. BY NODES
 BYNODE=.F. BY ELEMENTS
 METHOD BY WHICH HYDRAULIC PROPERTIES ARE GIVEN.....
 PERM=.T. PERMEABILITIES KXX,KYY ARE GIVEN
 PERM=.F. TRANSMISSIVITIES TXX,TYY ARE GIVEN

1
 BOUNDARY CONDITION PARAMETERS
 =====

NUMBER OF PRESCRIBED BOUNDARY HEAD NODES.....NHNODE= 22
 NUMBER OF TIMES WHEN PRESCRIBED HEAD BOUNDARY CONDITIONS ARE SET.....NTSH= 1
 NUMBER OF NODES WHERE GROUND LEVELS ARE SPECIFIED.....NBRD= 0
 NUMBER OF TIMES WHEN GROUND LEVELS ARE SET.....NTGRD= 0
 NUMBER OF FLUX ELEMENTS.....NFLXE= 0
 NUMBER OF TIMES WHEN PRESCRIBED BOUNDARY FLUXES ARE SET.....NTFLXE= 0
 NUMBER OF REGIONS WITH REGIONAL ELEMENT FLUX.....NEFR= 0
 NUMBER OF FLUX SIDES.....NFLXS= 0
 NUMBER OF TIMES WHEN PRESCRIBED SIDE FLUXES ARE SET.....NTFLXS= 0
 NUMBER OF FLUX NODES.....NFLXN= 1
 NUMBER OF TIMES WHEN PRESCRIBED NODE FLUXES ARE SET.....NTFLXN= 2

NUMBER OF 3RD-TYPE BOUNDARY SIDES.....NRCS= 0
 NUMBER OF TIMES WHEN 3RD-TYPE BOUNDARY HEADS ARE SET.....NTEC3= 0
 NUMBER OF NODES ON THE 3RD-TYPE BOUNDARY SIDES.....NNOD3= 0
 3RD-TYPE BOUNDARY PARAMETER.....REDECS= .F.
 BEDDC3=.T. USE RIVER BED LEVELS TO MODIFY 3RD-TYPE INFLOWS
 REDBC3=.F. DO NOT USE RIVER BED LEVELS
 NUMBER OF NODES WHERE HEADS IN ADJACENT AQUIFER ARE GIVEN.....NHADJ= 0
 NUMBER OF TIMES WHEN HEADS IN ADJACENT AQUIFER ARE SET.....NHADJ= 0
 ADJACENT AQUIFER PARAMETER.....ATTOP= .F.
 ATTOP=.T. ADJACENT AQUIFER IS ABOVE AQUIFER UNDER STUDY
 ATTOP=.F. ADJACENT AQUIFER IS NOT AT TOP
 METHOD OF SUPPLYING ADJACENT HEADS.....HAVARY= .F.
 HAVARY=.T. ADJACENT HEADS VARY IN SPACE
 HAVARY=.F. ADJACENT HEADS ARE CONSTANT
 TYPE OF INITIAL CONDITIONS/GUESSES IF INITIAL=.F.....ICVARY= .T.
 ICVARY=.T. INITIAL CONDITIONS VARY IN SPACE
 ICVARY=.F. INITIAL CONDITIONS ARE CONSTANT

SOLUTION STRATEGY

=====
 START TIME.....STRTIM= .0000
 END TIME.....ENDTIM= 100.0000
 TOLERANCE FOR CONVERGENCE.....TOL= .0500
 INDICATOR FOR CONVERGENCE CRITERION.....ITOL= 1
 ITOL=0 ROOT MEAN SQUARE ERROR CRITERION
 ITOL=1 MAXIMUM ABSOLUTE ERROR CRITERION
 MAXIMUM NUMBER OF ITERATIONS.....MAXIT= 5
 TIME STEP.....DT= .01000
 ALPHA PARAMETER FOR TIME INTEGRATION SCHEME.....ALPHA= .5000
 TIME STEP PARAMETER.....DTPARM= 1.1000
 INDICATOR FOR CHANGING TIME STEP.....IDT= 1
 IDT=0 NO CHANGE OF TIME STEP
 IDT=1 DT=DT*DTFARM AFTER NDT TIME STEPS
 IDT=2 DT=DT+DTPARM AFTER NDT TIME STEPS
 NUMBER OF CONSTANT LENGTH TIME STEPS BEFORE CHANGING ACCORDING TO IDT.....NDT= 1
 NUMBER OF ITERATIONS WITH SAME LEFT HAND SIDE SYSM MATRIX.....NSYS= 2
 NSYS=999 REFORM SYSM ONCE EACH TIME STEP

OUTPUT CONTROL

=====

FIRST TIME STEP FOR PRINTED OUTPUT.....ISTFRT= 1000000
NUMBER OF TIME STEPS BETWEEN PRINTED OUTPUTS.....NDTFRT= 1
NUMBER OF SPECIAL PRINTED OUTPUT TIMES.....NOUT= 2
FIRST TIME STEP FOR PLOTTED OUTPUT.....ISTFLT= 1000000
NUMBER OF TIME STEPS BETWEEN PLOTTED OUTPUTS.....NDTFLT= 1
NUMBER OF SPECIAL PLOTTED OUTPUT TIMES.....NFLT= 6
PLOT SCALING FACTOR.....FSCALE= 300.00
INCREMENT FOR LABELLING PLOT AXES.....FDelta= .00
ROTATION PARAMETER.....NRFLAG= 1
NRFLAG=0 ROTATION OF PLOT IS ALLOWED
NRFLAG=1 NO ROTATION IS ALLOWED, X-AXIS ACROSS THE PAGE
NRFLAG=2 ROTATION IS FORCED, Y-AXIS ACROSS THE PAGE
NUMBER OF ROWS/COLUMNS AROUND PLOT BOUNDARY.....NBOUND= 3
NUMBER OF DECIMAL PLACES IN PLOTTED VALUES OF HEAD.....NDF= 2

THE TOTAL MODEL AREA IS .8000E+06

1 NUMBER OF ELEMENTS (NK) IN SYMMETRIC COMPACTED MATRIX IS 4848
BANDWIDTH OF MATRIX IN FULL FORM WOULD BE 55

27432 WORDS OF STORAGE HAVE BEEN USED OUT OF A TOTAL ALLOCATION OF 50000
CALCULATIONS ARE IN SINGLE PRECISION

1 AQUIFER CHARACTERISTICS
=====

ELEMENT NO	BOTTOM ELEVATION	AQUIFER THICKNESS	KXX	KYY	STORATIVITY	SPECIFIC YIELD	CONF. LAYER K. / B.
1	20.00	50.00	5.000E+00	5.000E+00	.000E+00	1.000E-01	.000E+00
2	20.00	50.00	5.000E+00	5.000E+00	.000E+00	1.000E-01	.000E+00
3	20.00	50.00	5.000E+00	5.000E+00	.000E+00	1.000E-01	.000E+00
4	20.00	50.00	5.000E+00	5.000E+00	.000E+00	1.000E-01	.000E+00
5	20.00	50.00	5.000E+00	5.000E+00	.000E+00	1.000E-01	.000E+00
6	20.00	50.00	5.000E+00	5.000E+00	.000E+00	1.000E-01	.000E+00
7	20.00	50.00	5.000E+00	5.000E+00	.000E+00	1.000E-01	.000E+00
8	20.00	50.00	5.000E+00	5.000E+00	.000E+00	1.000E-01	.000E+00
9	20.00	50.00	5.000E+00	5.000E+00	.000E+00	1.000E-01	.000E+00
10	20.00	50.00	5.000E+00	5.000E+00	.000E+00	1.000E-01	.000E+00
11	20.00	50.00	5.000E+00	5.000E+00	.000E+00	1.000E-01	.000E+00
12	20.00	50.00	5.000E+00	5.000E+00	.000E+00	1.000E-01	.000E+00
13	20.00	50.00	5.000E+00	5.000E+00	.000E+00	1.000E-01	.000E+00
14	20.00	50.00	5.000E+00	5.000E+00	.000E+00	1.000E-01	.000E+00
15	20.00	50.00	5.000E+00	5.000E+00	.000E+00	1.000E-01	.000E+00

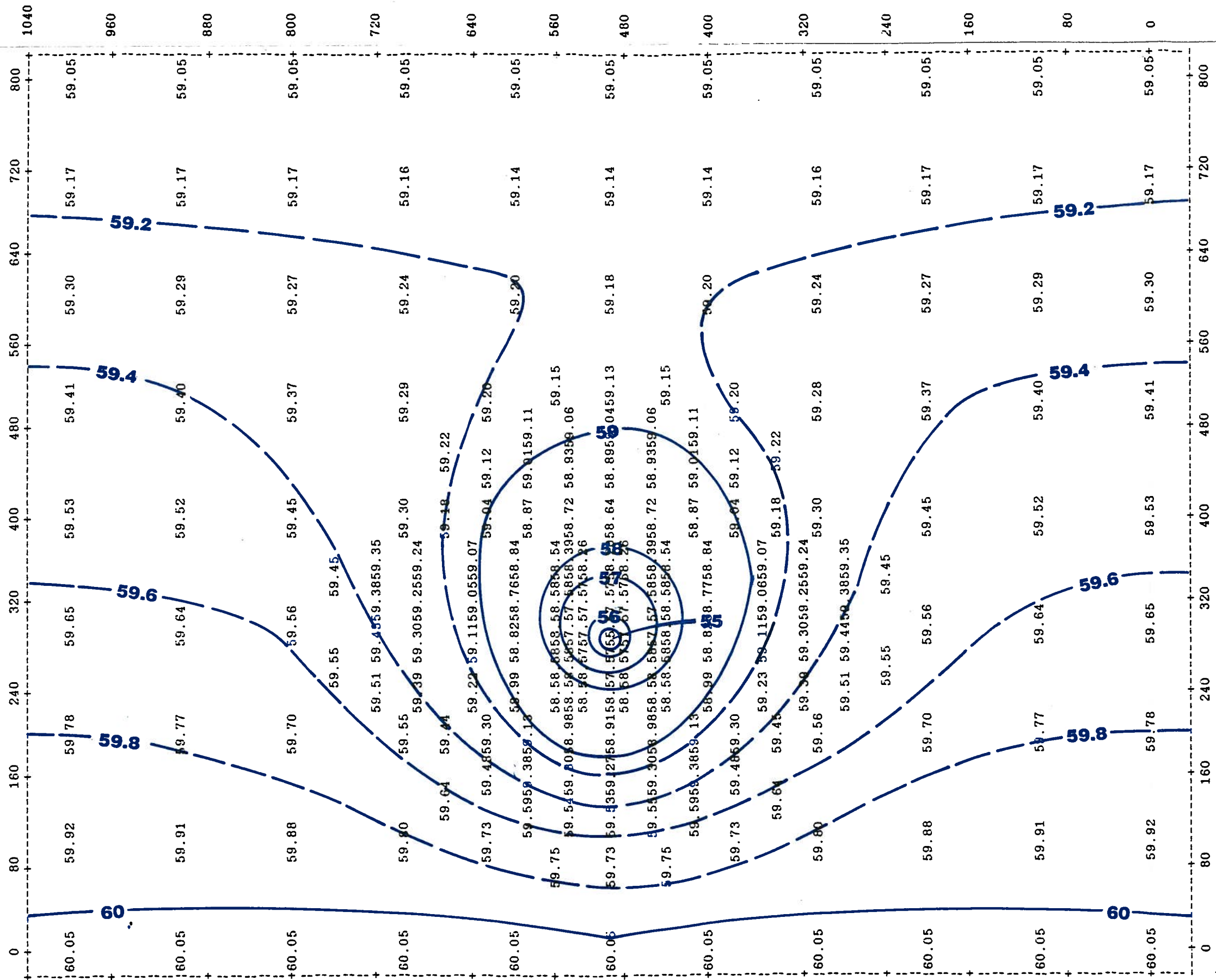
201 59.18 202 59.05 203 59.05 204 59.05 205 59.05 206 59.05 207 59.05 208 59.05 209 59.18 210 59.30
 211 59.43 212 59.55 213 59.68 214 59.68 215 59.18 216 59.05 217 59.18 218 59.05 219 59.05 220 59.05
 221 59.18 222 59.30 223 59.43

1 TABLE OF PRINTED OUTPUT TIMES
 =====

1 50.0000 100.0000

1 TABLE OF PRINT-PLOT TIMES
 =====

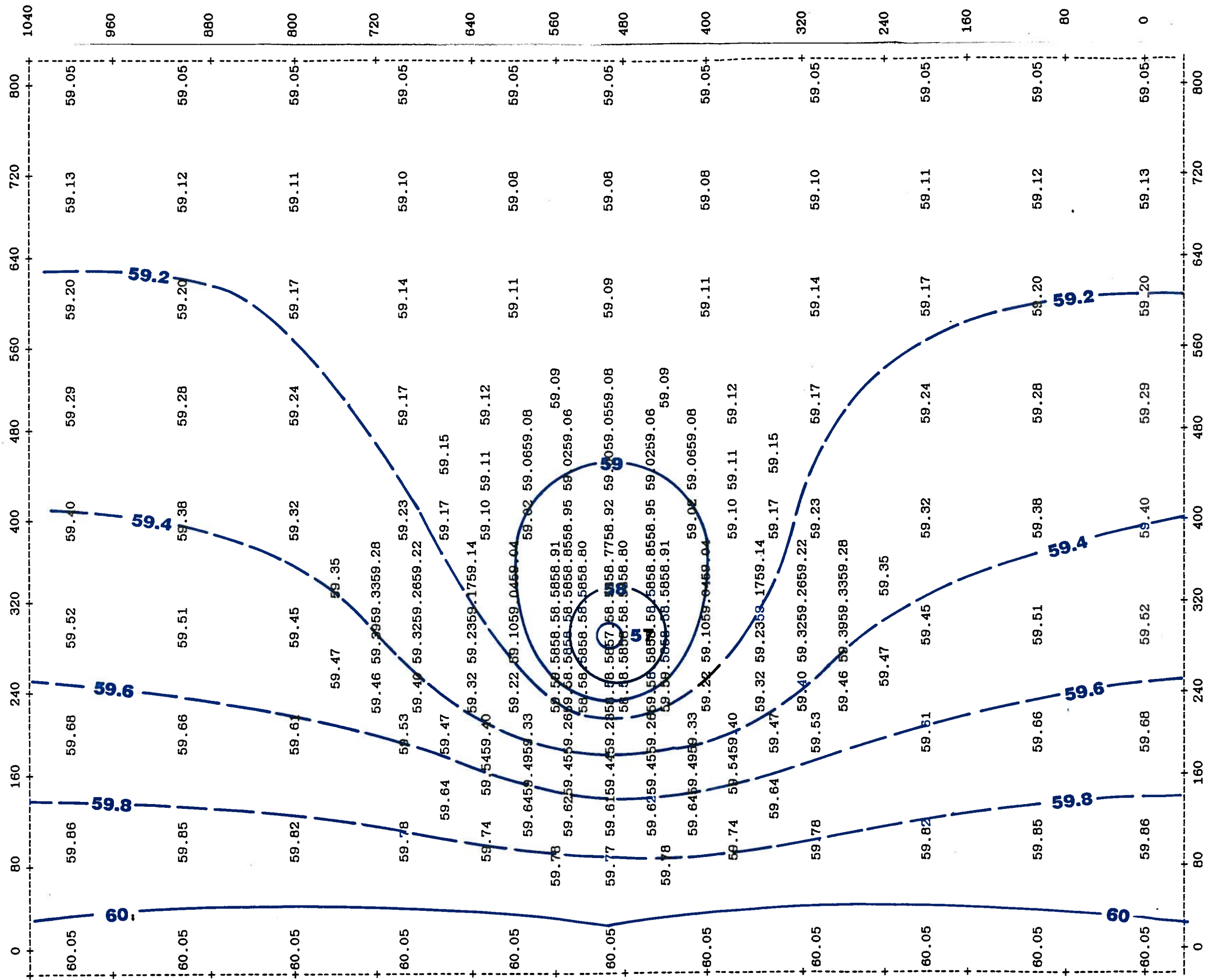
1 20.0000 40.0000 50.0000 60.0000 80.0000 100.0000



Aquifem Well Drawdown Model
 Pumping Well 86-2 at 4gpm K=3
 Time = 20.00000
 X Coordinates Range from .00 to 800.0
 Y Coordinates Range from .00 to 1000.
 Scale: 1/3000



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Aquifer Well Drawdown Model:
Pumping Well 86-2 at 8 gpm K=15

Time = 20.00000

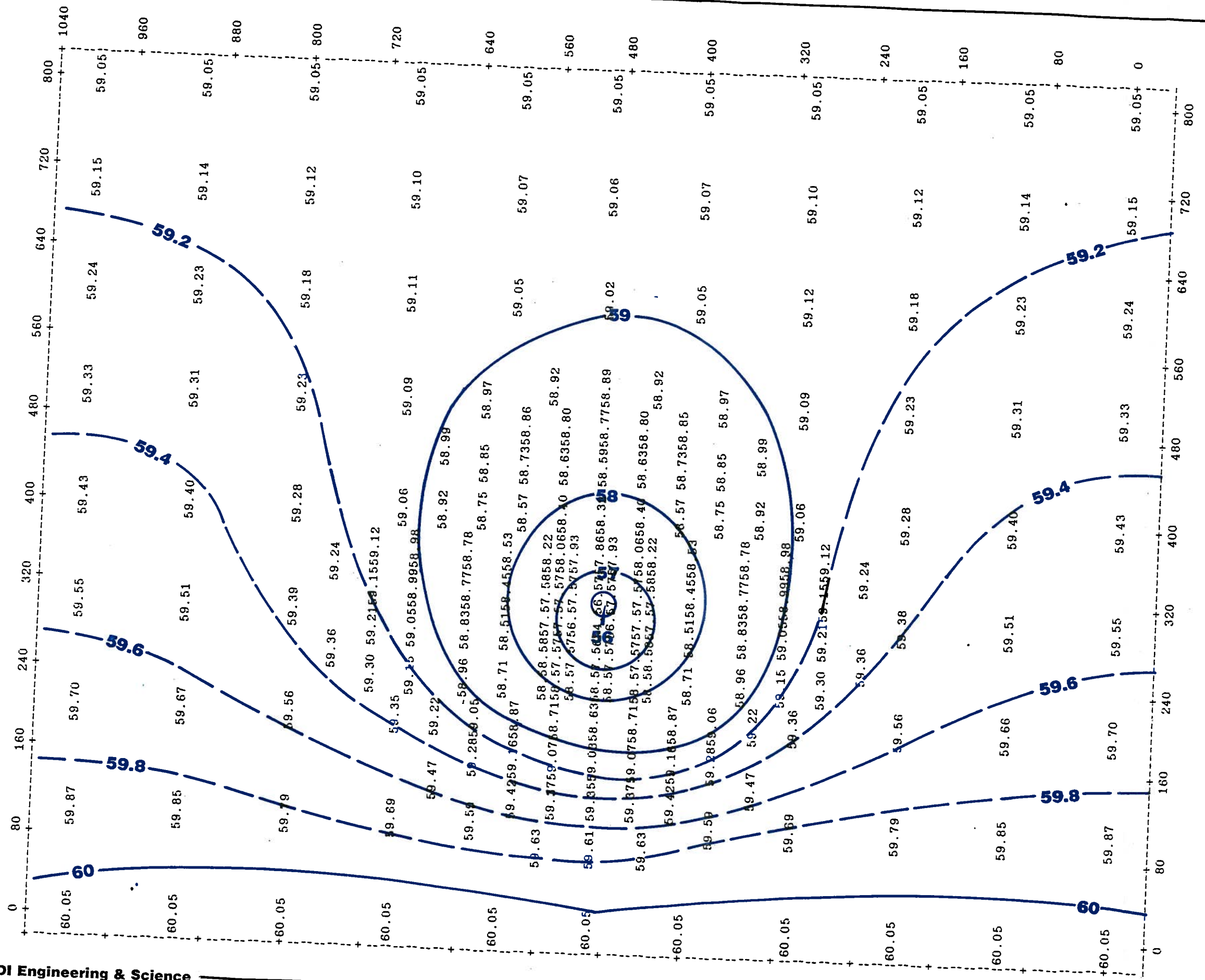
X Coordinates Range from .00 to 800.00

Y Coordinates Range from .00 to 1000.0

Scale: 1/3000



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Aquifer Well Drawdown Mod
 Pumping Well 86-2 at 8gpm K=6

Time = 20.00000

X Coordinates Range from .00 to 800

Y Coordinates Range from .00 to 1000

Scale: 1/3000



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